

# CERTIFICATE of APPRECIATION

THIS ACKNOWLEDGES THAT

Andy Prabowo, Ph.D.

Has been invited as **A GUEST SPEAKER** in the event of:

Academic License Webinar: SIMULIA Abaqus & CST Studio Suite

December 13<sup>th</sup>, 2023



Muhammad Badruddin  
Advanced Analysis Group Worley SEA Indonesia





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# Numerical Model of Perforated Stainless Steel Beam under Four-Point Bending

Webinar Abaqus December 13<sup>th</sup>, 2023

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Civil Engineering Department UNTAR

# Application of Abaqus in UNTAR's Civil Engg Dept

- Abaqus has been procured at the End of 2022 through Worley Indonesia
- Current version: Abaqus 2023 (1 perpetual license)
- Application for research area:
  - ✓ Finite element model of cold-formed steel member behaviour: column, beam, shear, web crippling, and others
  - ✓ Considered at ambient and elevated temperatures
  - ✓ Future plan: direct analysis method, connection, earthquake engg



ELSEVIER

Contents lists available at ScienceDirect

# Thin-Walled Structures

journal homepage: [www.elsevier.com/locate/tws](http://www.elsevier.com/locate/tws)



Full length article

## Cold-formed stainless steel beams with single web hole at elevated temperatures

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### ARTICLE INFO

#### Keywords:

Cold-formed stainless steel  
Direct strength method  
Elevated temperatures  
Perforated beams

### ABSTRACT

Steel structures are often used in buildings due to their advantage in weight-to-strength ratio. However, their structural capacity deteriorates in fire as the temperature of the structures rises. Investigation of cold-formed stainless steel (CFSS) structures at elevated temperatures is still limited, especially for rectangular hollow section (RHS) beams having a single web hole in the mid-span (perforated web). Therefore, a numerical investigation was conducted to evaluate the current design provisions to calculate the strength of such beams at

Source title ↓

Documents  
2019-22 ↓

% Cited ↓

SNIP ↓

SJR ↓

Publisher ↓



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Thin-Walled Structures

2,714

83

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1.434

Elsevier

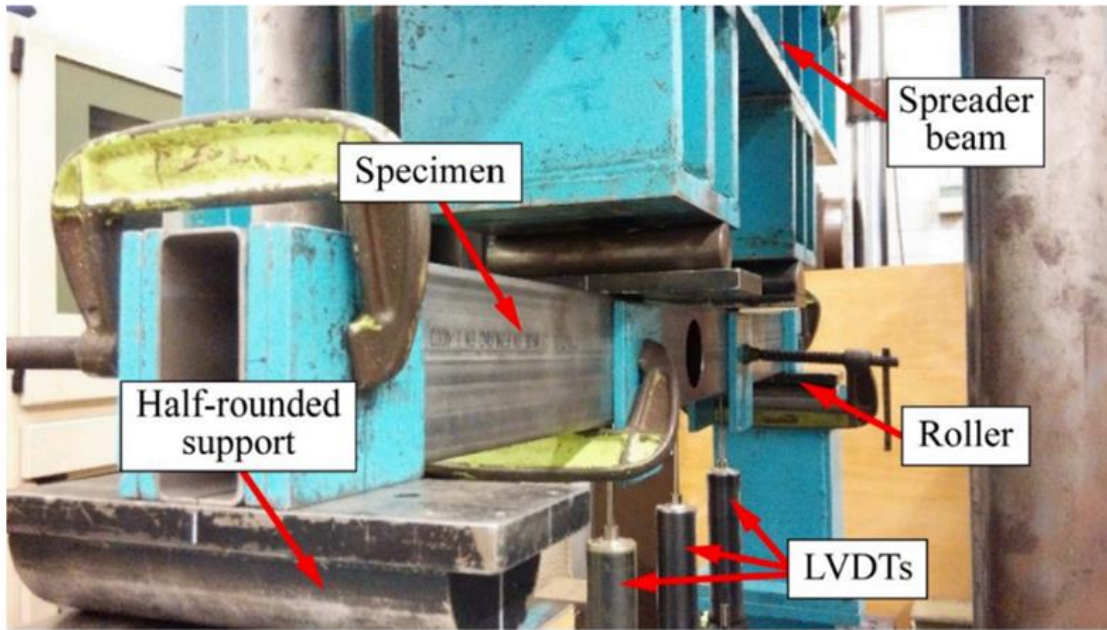


Fig. 1. Test setup of perforated RHS beam (Chen et al., 2022)

# The Problem

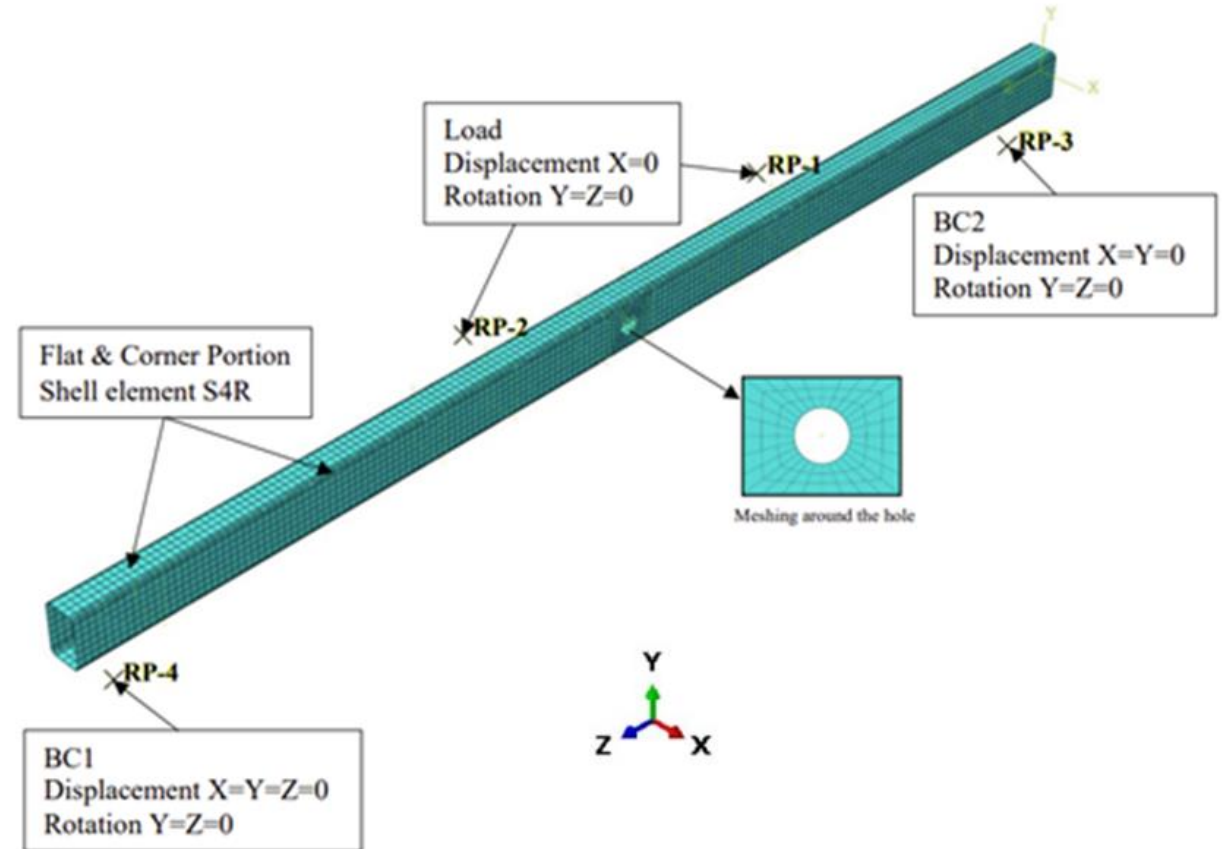


Fig. 2. Finite element model of perforated RHS beam (Prabowo et al., 2023)

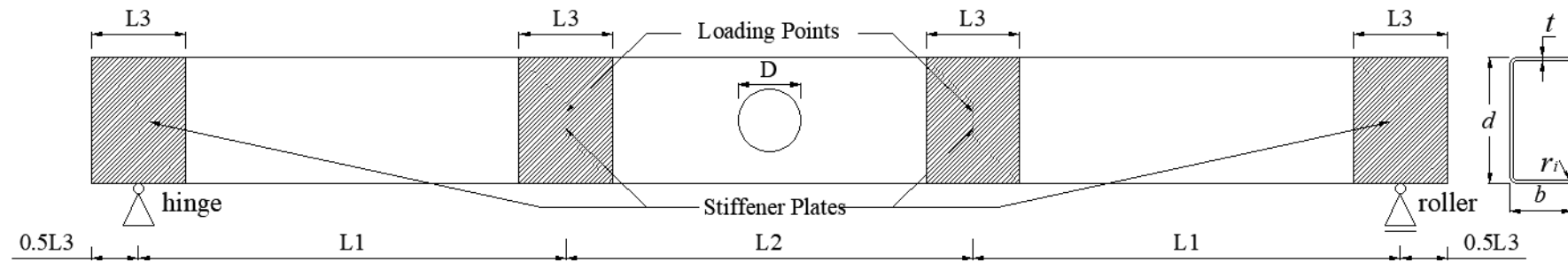
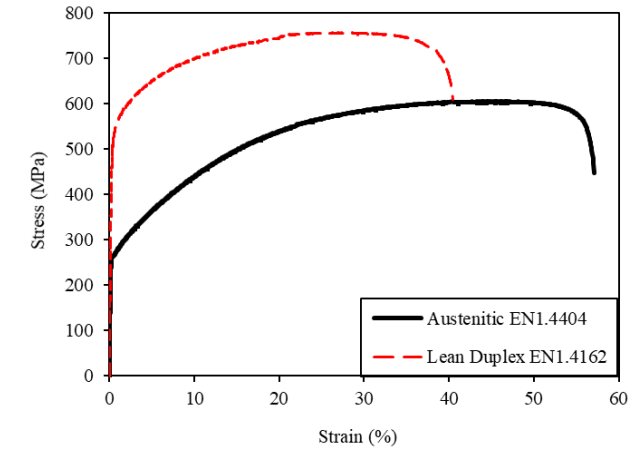
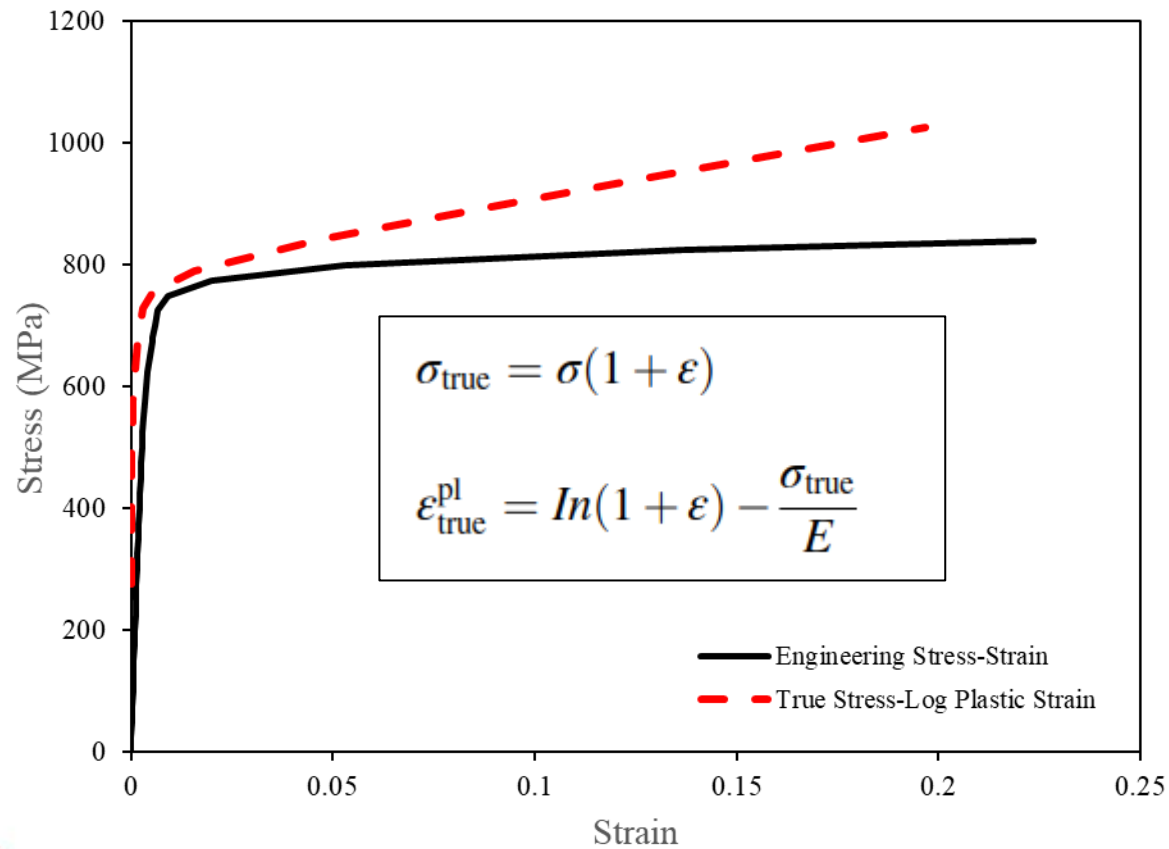


Fig. 3. Dimension of specimens

Specimen	$L$ (mm)	$H$ (mm)	$B$ (mm)	$t$ (mm)	$r_o$ (mm)	$r_i$ (mm)	$D$ (%)
60×40×4-D0	1301.0	60.07	40.24	3.87	7.63	4.44	0.00
60×40×4-D20	1290.0	60.08	40.44	3.96	7.63	4.44	20.30
60×40×4-D50	1300.5	59.88	40.21	3.90	7.63	4.44	50.78
60×40×4-D80	1301.0	60.05	40.17	3.79	7.63	4.44	80.12
80×60×4-D0	1301.5	80.35	60.31	3.74	8.38	4.94	0.00
80×60×4-D20	1299.0	80.51	60.15	3.79	8.38	4.94	19.54
80×60×4-D50	1299.6	80.32	60.13	3.77	8.38	4.94	49.58
80×60×4-D50 <sup>+</sup>	1299.0	80.33	60.33	3.93	8.38	4.94	49.19
80×60×4-D80	1298.5	80.36	60.14	3.82	8.38	4.94	78.88

# Material Nonlinearity



# Boundary conditions

- BCs were set based on DOF activation
- Replicating BCs from the experiment setup is an important setting (modelled through loading setup)
- FEA using Displacement control method → target displacements were keyed

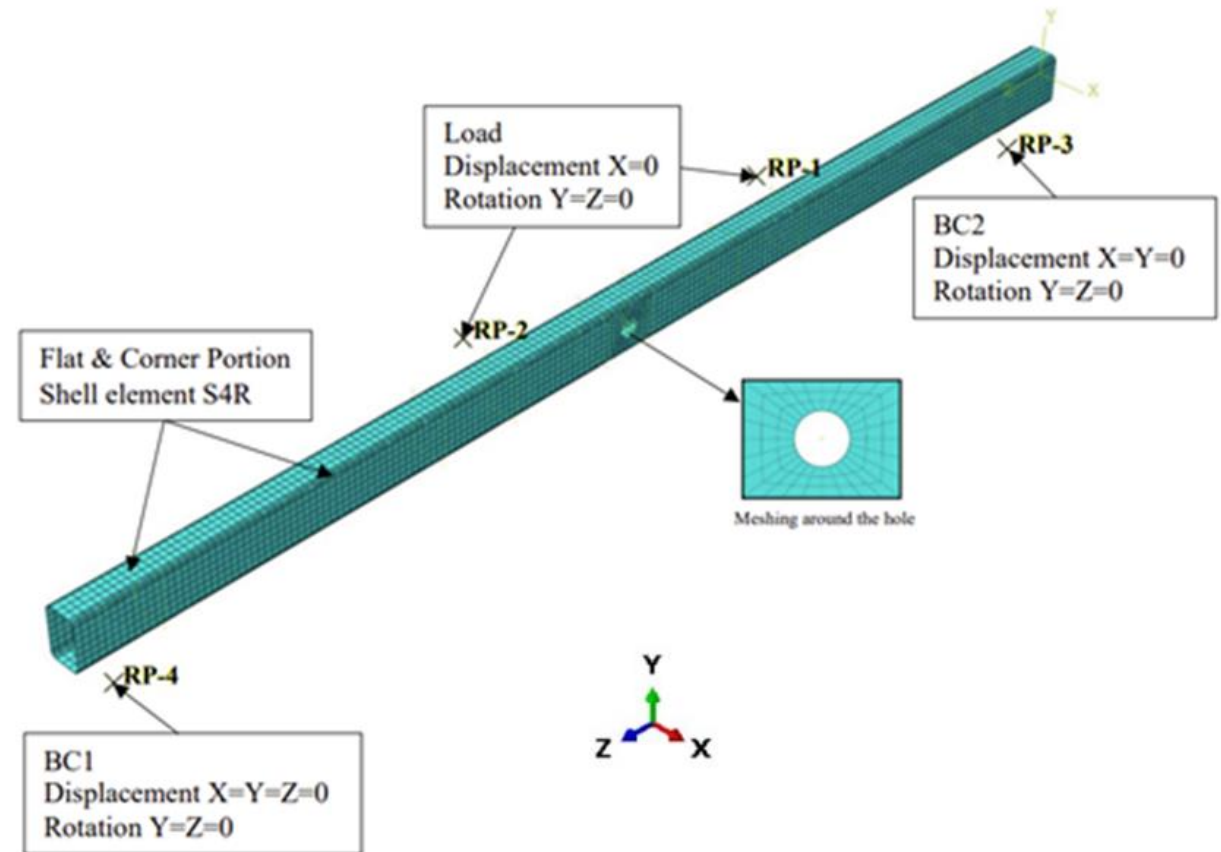


Fig. 2. Finite element model of perforated RHS beam (Prabowo et al., 2023)



# Boundary Conditions

**Edit Boundary Condition**

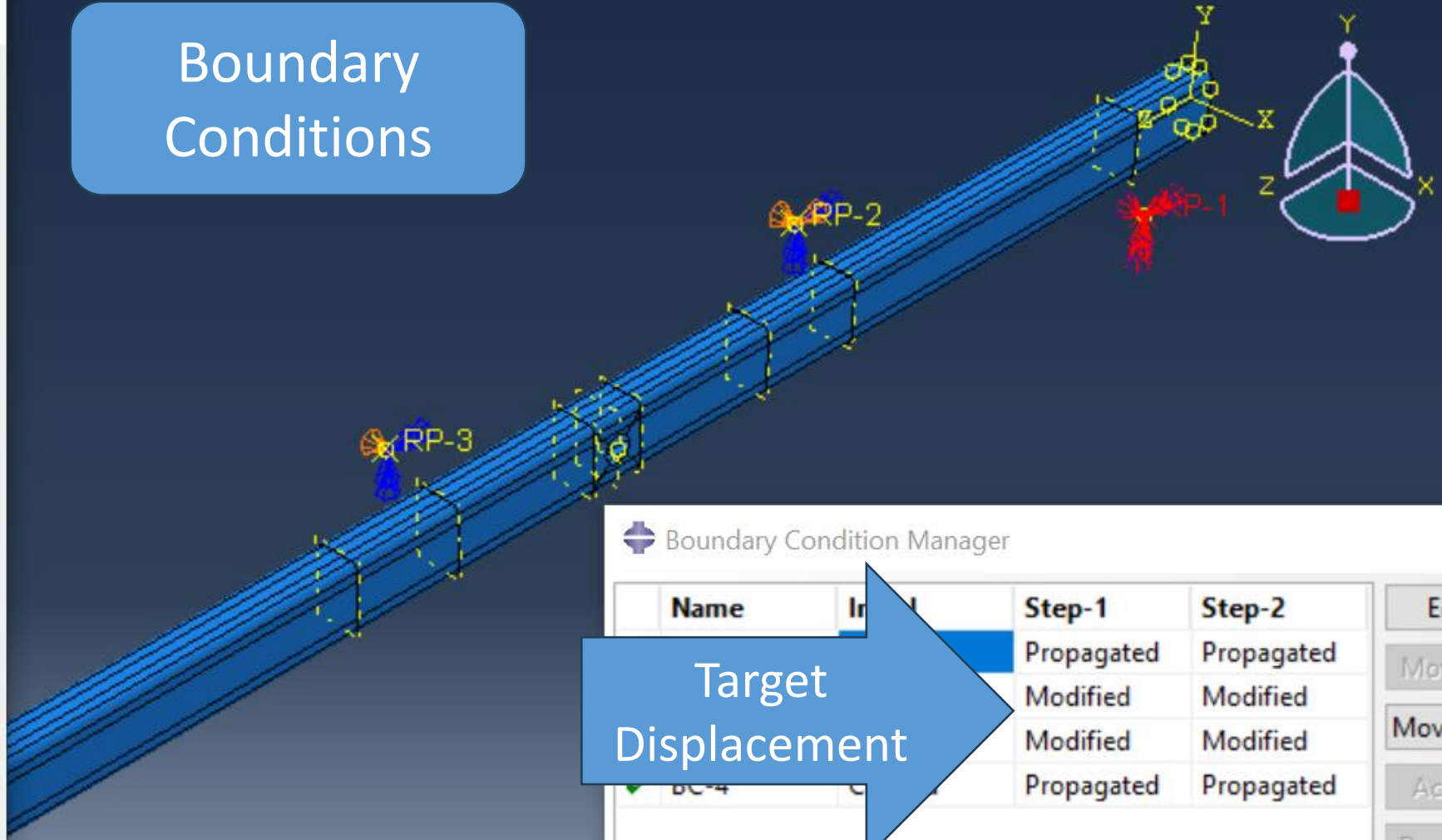
Name: BC-1  
Type: Displacement/Rotation  
Step: Initial  
Region: Set-5

CSYS: (Global)

- U1
- U2
- U3
- UR1
- UR2
- UR3

**Note:** The displacement value will be maintained in subsequent steps.

OK Cancel



**Boundary Condition Manager**

Name	Initial	Step-1	Step-2	Edit...
		Propagated	Propagated	Move Left
		Modified	Modified	Move Right
		Modified	Modified	Activate
		Propagated	Propagated	Deactivate

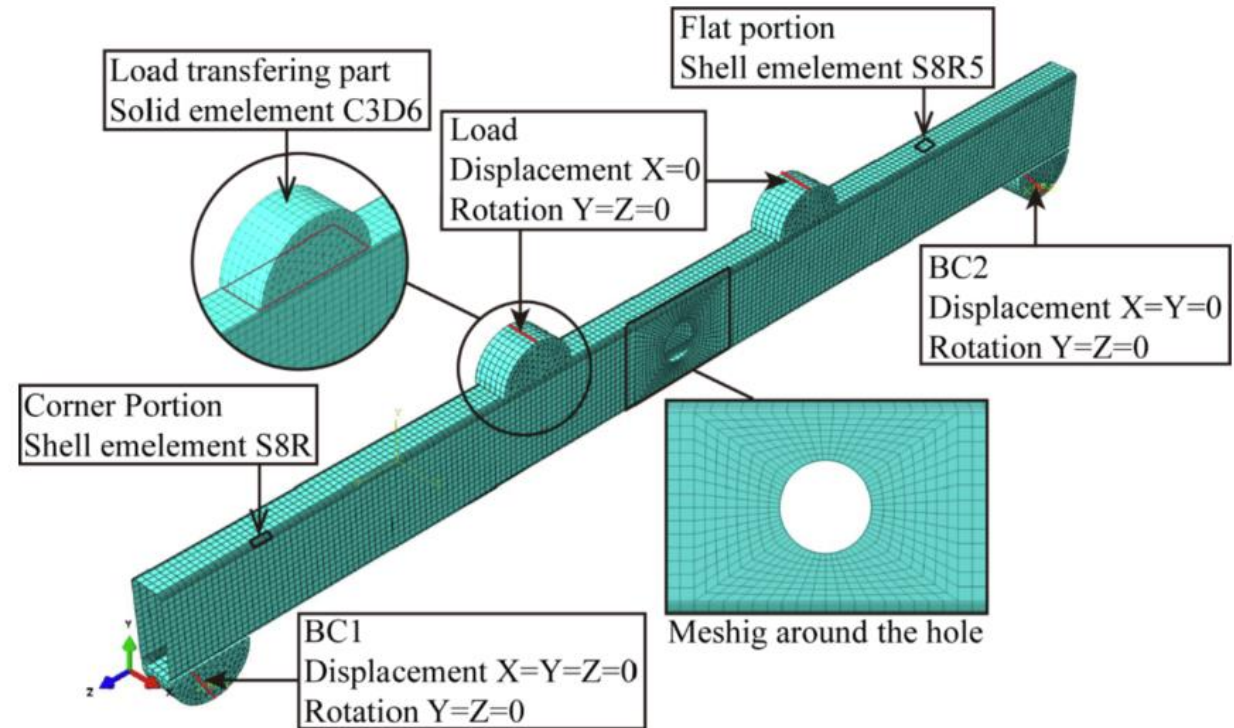
Step procedure:  
Boundary condition type: Displacement/Rotation  
Boundary condition status: Created in this step

Create... Copy... Rename... Delete... Dismiss

Fill out the Edit Boundary Condition dialog

# Finite Element Meshes

- Cold-formed steel member → shell element
- Element type: S4R, S8R, S8R5
- Loading block → solid element
- Interaction between elements should be modelled carefully



# Interaction Module

Edit Constraint

Name: Constraint-1

Type: Coupling

Control points: m\_Set-1

Surface: s\_Surf-1

Coupling type:  Kinematic  
 Continuum distributing  
 Structural distributing

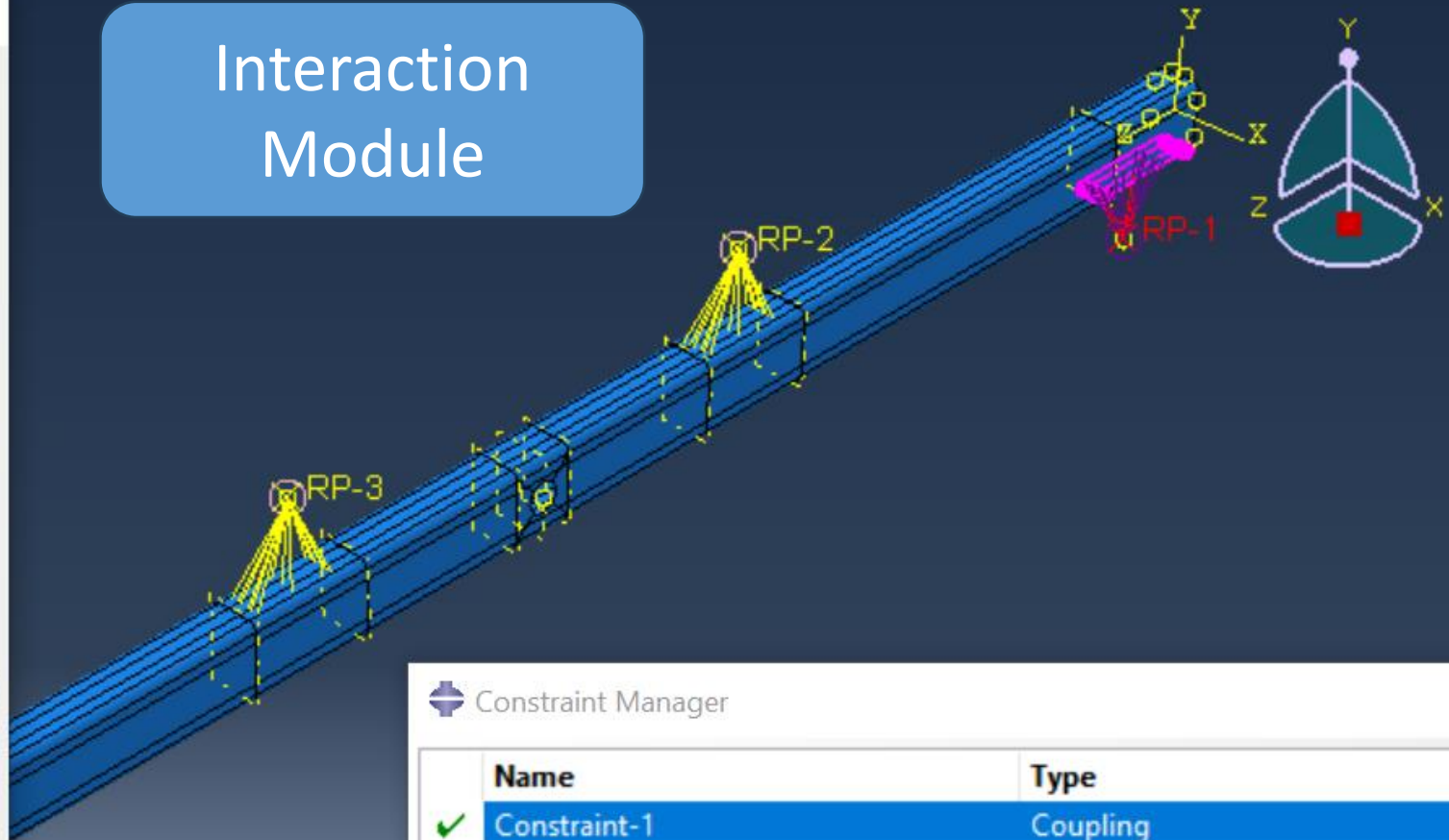
Constrained degrees of freedom:  
 U1  U2  U3  UR1  UR2  UR3

Influence radius:  To outermost point on the region  
 Specify:

Adjust control points to lie on surface

CSYS (Global)

OK Cancel



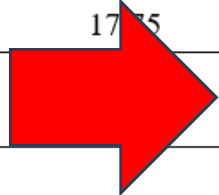
Constraint Manager

Name	Type
✓ Constraint-1	Coupling
✓ Constraint-2	Coupling
✓ Constraint-3	Coupling
✓ Constraint-4	Coupling

Create... Edit... Copy... Rename... Delete... Dismiss

# FEM Validations – Comparison of ultimate strengths

Specimen ( $d \times b \times t$ )	$D/h$ (%)	$M_{Test}$ (kNm)	$M_{FEA}$ (kNm)	$M_{Test}/M_{FEA}$	$K_{Test} (.10^{-4})$	$K_{FEA} (.10^{-4})$	$K_{Test}/K_{FEA}$
60×40×4	0	7.59	7.41	1.02	4.92	5.13	0.96
	20	7.54	7.40	1.02	4.89	5.12	0.95
	50	7.12	6.76	1.05	2.11	3.00	0.70
	80	6.23	6.21	1.00	1.52	1.67	0.91
80×60×4	0	14.49	13.90	1.04	3.23	3.10	1.04
	20	14.43	13.73	1.05	3.18	3.10	1.03
	50	13.67	13.38	1.02	1.32	1.58	0.83
	50 (r)	13.88	13.38	1.04	1.41	1.58	0.89
	80	12.28	11.95	1.03	0.96	1.12	0.85
100×40×2	0	8.32	7.83	1.06	1.22	2.35	0.52
	20	8.2	7.88	1.04	1.28	1.82	0.71
	50	7.40	7.22	1.02	0.82	0.94	0.87
	50 (r)	7.57	7.22	1.05	0.84	0.94	0.89
	80	6.15	5.83	1.05	0.66	0.66	1.00
120×80×3	0	21.63	20.16	1.07	0.81	1.21	0.67
	20	21.83	20.14	1.08	0.80	1.21	0.66
	50	20.26	19.05	1.06	0.54	1.13	0.48
	80	17.75	16.04	1.11	0.47	0.47	0.98
				Mean	1.05		0.83
				COV	0.019		0.204



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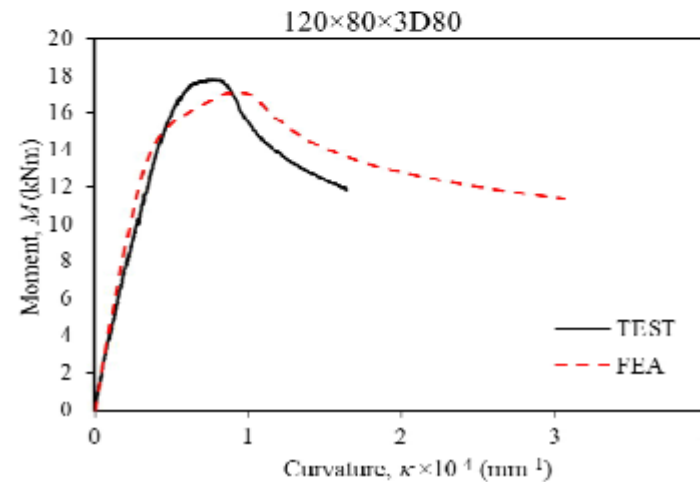
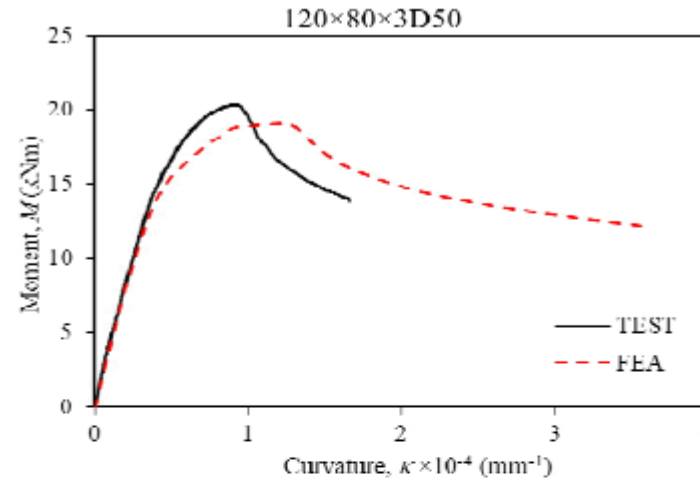
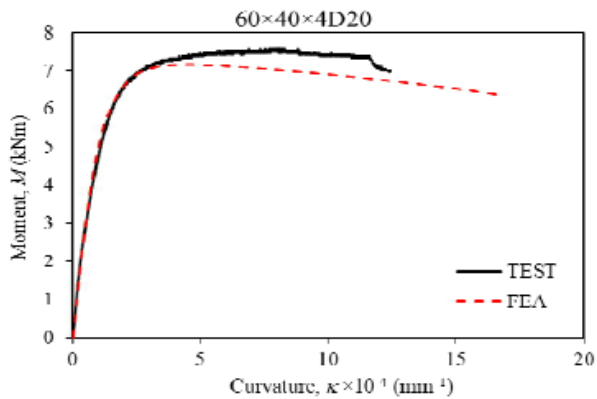
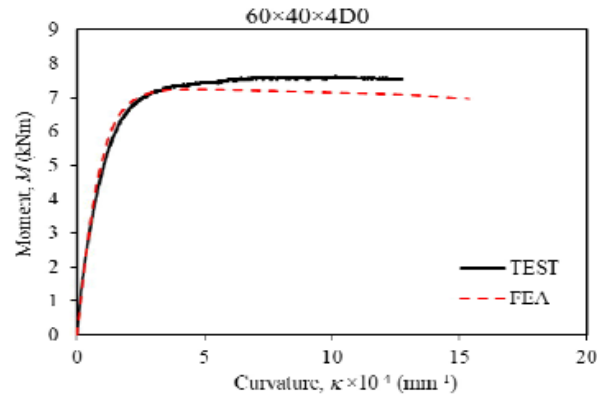


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# FEM Validations – Comparison of Moment vs Curvature Curves



# FEM Validations – Failure modes



From the three comparisons, it was concluded that the FE model developed in Abaqus could predict the test results well

# Parametric study conducted reported in the journal

Parameter variations in the parametric study.

Parameters	Austenitic & Lean duplex
Sections ( $d \times b \times t$ )	60×40×4, 120×80×3, 300×120×4.5, 380×286×2, 380×152×1.5, 380×380×4, 380×570×4, 380×570×2
Web slenderness ( $h/t$ )	10.8 – 246.7
Inner radius to thickness ratio ( $r_i/t$ )	0.9 – 2.3
Hole diameter to web depth ratio ( $D/h$ )	0, 20%, 50%, 70%, 90%
Elevated temperatures (°C)	22, 320, 550, 660, 870 (Austenitic) 24, 300, 500, 700, 900 (Lean duplex)



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# Parametric Study

Flexural strength values (kNm) obtained from FEA.

Cross-section			T ( °C)	Austenitic (EN 1.4301)					T ( °C)	Lean duplex (EN 1.4162)				
d (mm)	b (mm)	t (mm)		D0	D20	D50	D70	D90		D0	D20	D50	D70	D90
60.07	40.24	3.87	22	7.1	6.9	6.2	5.8	5.4	24	10.3	10.1	9.4	8.9	8.1
			320	5.8	5.6	5.0	4.6	4.3	300	8.8	8.5	7.8	7.4	6.7
			550	5.4	5.2	4.6	4.2	3.9	500	7.7	7.5	6.7	6.2	5.7
			660	4.2	4.1	3.7	3.5	3.2	700	3.1	3.1	2.9	2.7	2.5
			870	0.9	0.9	0.9	0.9	0.8	900	0.8	0.8	0.8	0.7	0.7
120.02	80.3	2.89	22	18.5	18.4	17.6	15.5	13.1	24	30.3	30.3	29.0	26.5	21.2
			320	13.3	13.3	12.9	11.4	9.3	300	23.4	23.4	23.0	21.2	14.9
			550	11.4	11.4	11.0	9.7	7.9	500	18.0	18.0	17.7	16.1	11.3
			660	10.5	10.5	10.1	9.0	7.3	700	9.0	9.1	8.9	7.8	6.3
			870	3.2	3.2	3.0	2.7	2.3	900	2.5	2.5	2.4	2.2	1.8
300	120	2	22	32.2	31.4	29.9	28.2	24.1	24	48.7	47.3	45.4	43.0	36.2
			320	26.4	26.0	24.5	23.4	19.4	300	37.9	36.8	35.4	33.5	28.3
			550	22.8	22.6	21.4	20.1	17.1	500	28.9	28.1	27.0	25.5	21.5
			660	19.3	19.1	18.2	17.1	14.5	700	16.3	16.0	15.4	14.4	12.3
			870	10.0	9.5	8.1	7.8	6.2	900	5.4	5.4	5.0	4.8	4.0
300	206	2	22	47.1	46.4	44.5	43.2	36.4	24	73.4	68.5	67.4	65.4	54.1
			320	40.1	37.3	36.0	33.6	29.2	300	57.0	53.4	52.3	50.3	42.2
			550	34.4	32.1	31.0	28.4	25.1	500	43.8	40.8	39.5	38.4	32.3
			660	28.0	27.8	26.0	25.0	21.7	700	25.3	23.7	23.0	21.7	18.7
			870	13.1	12.1	11.5	10.7	10.7	900	8.1	7.5	7.2	6.5	5.7
300	152	1.5	22	26.8	26.7	26.6	23.9	20.3	24	40.1	39.4	37.5	36.0	35.7
			320	21.8	21.1	20.4	18.7	15.9	300	31.3	30.6	29.0	28.0	24.6
			550	18.8	18.3	18.2	16.1	13.7	500	23.7	23.3	22.2	21.4	18.7
			660	15.8	15.4	14.5	13.9	11.8	700	13.4	13.3	12.7	12.0	10.3
			870	7.2	6.8	6.8	6.0	5.7	900	4.4	4.2	4.2	3.8	3.2
300	300	4	22	169.6	167.5	157.5	144.4	128.2	24	290.5	289.5	274.9	219.2	192.7
			320	135.6	131.5	123.2	112.7	111.9	300	226.1	226.1	215.6	197.5	178.2
			550	122.0	115.0	104.6	97.1	81.6	500	174.2	174.5	142.8	134.8	114.2
			660	115.3	106.0	104.4	85.3	73.2	700	110.3	109.3	104.8	86.6	84.8
			870	43.1	41.7	38.8	35.7	35.6	900	33.8	33.7	32.5	30.8	25.7
300	570	4	22	196.5	195.1	169.5	153.5	136.9	24	283.8	282.5	253.7	230.8	209.8
			320	161.3	160.3	133.2	123.8	107.0	300	220.8	221.2	197.0	184.1	164.4
			550	139.2	138.4	115.2	106.7	91.9	500	171.0	169.9	150.7	141.3	125.2
			660	118.9	107.4	100.2	91.9	79.3	700	101.1	100.4	85.3	79.7	68.9
			870	50.9	50.4	43.0	38.9	33.7	900	32.5	32.2	27.0	24.7	21.4
300	570	2	22	54.4	54.4	53.0	52.6	47.9	24	78.0	77.9	77.4	75.6	69.3
			320	44.6	44.6	43.8	42.7	38.7	300	60.8	61.4	60.4	59.1	54.4
			550	38.5	38.4	37.6	36.9	33.3	500	47.0	47.3	45.6	45.6	42.2
			660	33.1	33.0	32.7	31.8	28.8	700	27.8	28.0	27.7	26.9	24.4
			870	14.9	14.8	14.6	14.0	12.3	900	8.9	9.0	8.7	8.6	7.7



# Some of key findings

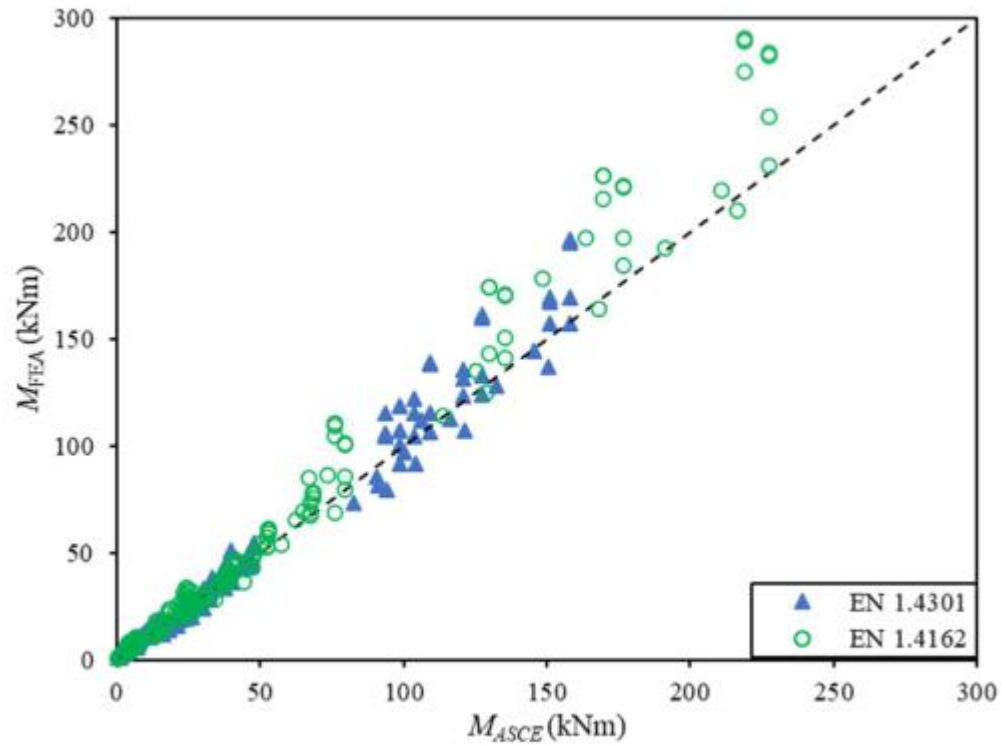


Fig. 13. Comparison of nominal flexural strengths obtained from FEA and ASCE [18] at various temperatures.

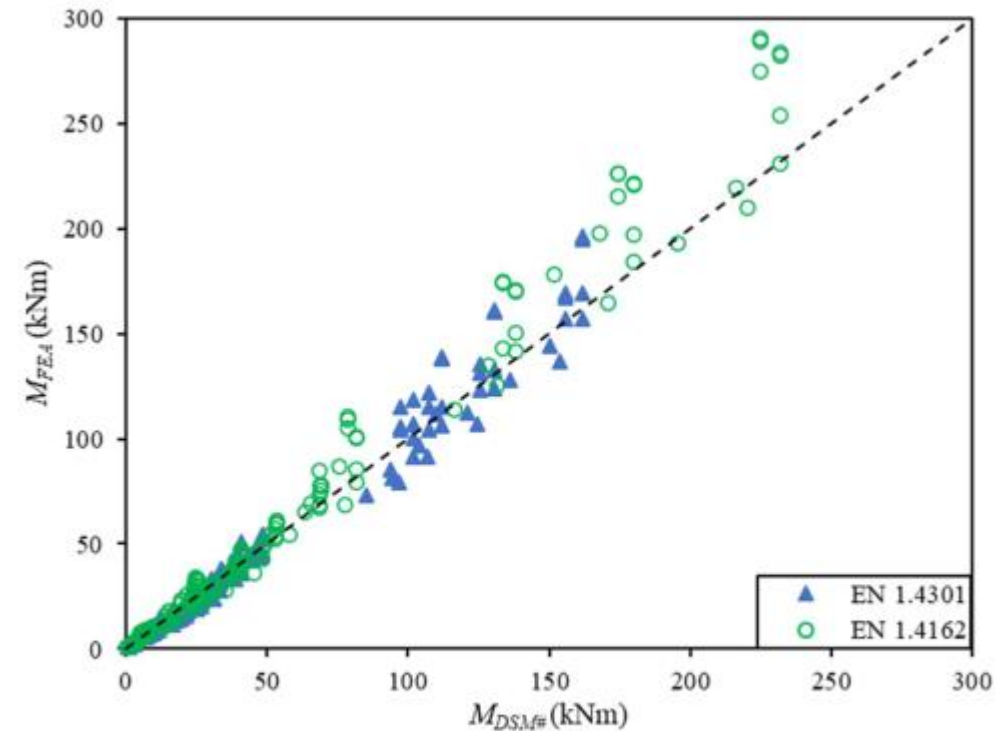


Fig. 14. Comparison of nominal flexural strengths obtained from FEA and modified DSM equations [22] at various temperatures.

# Results from Reliability Analyses

**Table 5**

Comparison between FEA results with nominal strengths predicted from design rules for cold-formed stainless steel RHS beams without a web hole (sections with  $D = 0$ ).

	$M_{FEA}/M_{ASCE}$	$M_{FEA}/M_{DSM\#}$	$M_{FEA}/M_{AS/NZS}$	$M_{FEA}/M_{EC3}$	$M_{FEA}/M_{EC3\#}$
<b>Austenitic (EN 14301)</b>					
Number of data	40	40	40	40	40
Mean ( $P_m$ )	1.24	1.11	1.16	1.26	1.41
COV ( $V_P$ )	0.249	0.146	0.283	0.129	0.202
Resistance factor ( $\phi$ )	0.90	0.90	0.9	0.91	1.00
Reliability index ( $\beta_o$ )	2.73	3.03	2.22	3.39	2.99
% ratio < 1	15%	30%	23%	0%	8%
Smallest ratio	0.93	0.90	0.81	1.00	0.89
Kruppa criteria	Passed	Failed	Failed	Passed	Passed
<b>Lean duplex (EN 1.4162)</b>					
Number of data	40	40	40	40	40
Mean ( $P_m$ )	1.23	1.14	1.17	1.32	1.43
COV ( $V_P$ )	0.178	0.115	0.199	0.086	0.210
Resistance factor ( $\phi$ )	0.90	0.90	0.9	0.91	1.00
Reliability index ( $\beta_o$ )	2.87	2.97	2.39	3.53	2.68
% ratio < 1	2.5%	17.5%	20%	0%	8%
Smallest ratio	0.98	0.95	0.87	1.14	0.93
Kruppa criteria	Passed	Passed	Failed	Passed	Passed

**Table 6**

Comparison between FEA results with nominal strengths predicted from design rules for cold-formed stainless steel RHS beams with a web hole (sections with  $D > 0$ ).

	$M_{FEA}/M_{ASCE}$	$M_{FEA}/M_{DSM\#}$	$M_{FEA}/M_{AS/NZS}$	$M_{FEA}/M_{EC3}$	$M_{FEA}/M_{EC3\#}$
<b>Austenitic (EN 14301)</b>					
Number of data	160	160	160	160	160
Mean ( $P_m$ )	1.12	0.99	1.17	1.22	1.34
COV ( $V_P$ )	0.252	0.150	0.207	0.152	0.189
Resistance factor ( $\phi$ )	0.90	0.90	0.90	0.91	1.00
Reliability index ( $\beta_o$ )	2.46	2.61	2.69	3.18	2.93
% ratio < 1	42%	59%	18%	3%	9%
Smallest ratio	0.73	0.70	0.85	0.94	0.88
Kruppa criteria	Failed	Failed	Passed	Passed	Passed
<b>Lean duplex (EN 1.4162)</b>					
Number of data	160	160	160	160	160
Mean ( $P_m$ )	1.13	1.03	1.19	1.26	1.36
COV ( $V_P$ )	0.195	0.136	0.154	0.120	0.213
Resistance factor ( $\phi$ )	0.90	0.90	0.90	0.91	1.00
Reliability index ( $\beta_o$ )	2.50	2.48	2.72	3.16	2.52
% ratio < 1	31%	49%	13%	3%	8%
Smallest ratio	0.76	0.74	0.89	0.98	0.87
Kruppa criteria	Failed	Failed	Passed	Passed	Passed

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# Thank you

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